GenCade Background



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CIRP Training on CMS and GenCade, Sept. 19-23, 2022



US Army Corps of Engineers®





Outline

Capabilities in GenCade:

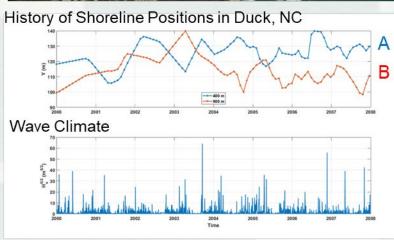
- Longshore Sediment Transport,
- Shoreline Change,
- Structures,
- Beach Fill and Bypassing
- Inlet Reservoir Model (IRM) for Sand Bypassing across Inlets



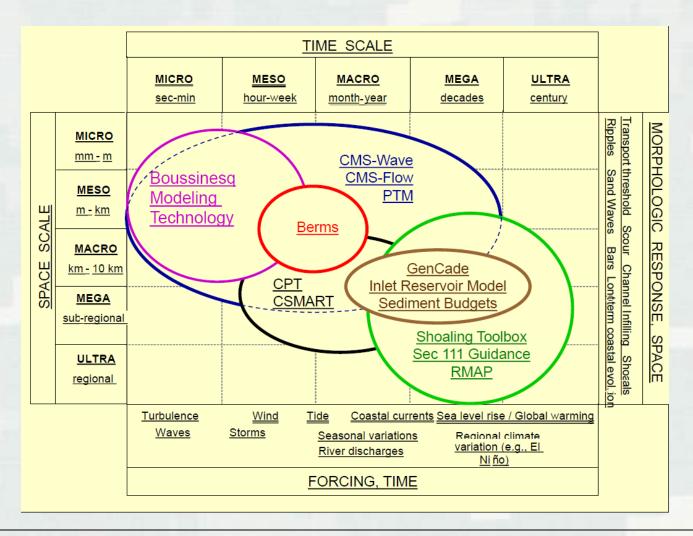
Long-Term and Regional Shoreline Changes

- Prediction of long-term shoreline changes is a key task in coastal management practices.
- Traditional shoreline evolution modeling is mainly based on the mass conservation of longshore sediment transport driven by waves and currents.
- Cross-shore transport also plays an important role in shoreline erosion and recovery (accretion)
- Physical processes driving cross-shore transport are complicated, including waves, currents (undertow), gravitational effects, sea level changes (tides, surges, and SLR), tectonic movement (subsidence), dune erosion due to aeolian transport, berm change, etc.
- Cross-shore sediment transport may play a dominant role in reshaping cross-shore profiles and shorelines in an initial stage of beach fills and nourishment.
- Shoreline changes induced by physical processes in general are highly irregular.





Scales and Model Coverage



Shoreline Change and Coastal Protection Practices

- Construction or modification of inlets for navigational purpose
- Construction of harbors with breakwaters built in nearshore regions
- Beachfills (sand nourishment)
- Sand Bypass
- Sand Mining
- Dredging Material Disposals



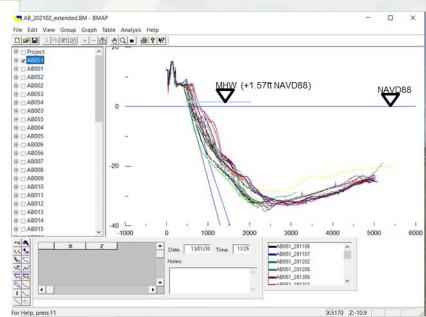


Shoreline Positions

Extract shoreline positions at a vertical datum (e.g. MHW) from transect survey data



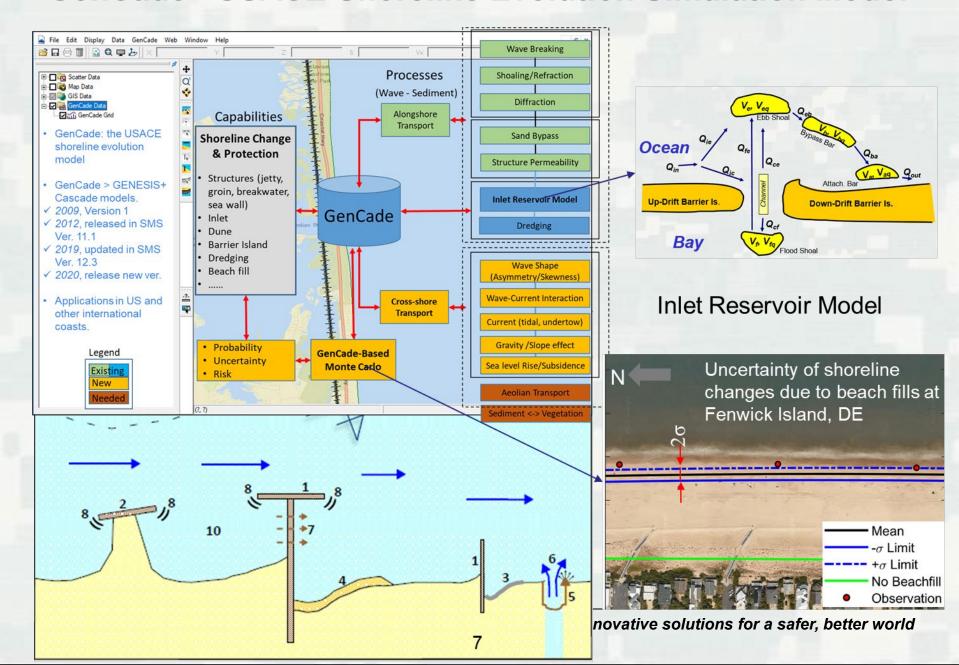
(a) Shore-normal transects on satellite imagery



(b) Profiles on BMAP



GenCade - USACE Shoreline Evolution Simulation Model



New Version of GenCade for Shoreline Evolution Modeling

Shoreline Change Equation with Sea Level Rise (SLR)

$$\frac{\partial y}{\partial t} + \frac{1}{D_s} \left(\frac{\partial Q}{\partial x} - q - \phi \right) + \frac{R + S}{\tan \beta} = 0$$

 ϕ : Cross-shore sediment transport rate

R: Sea Level Change Rate

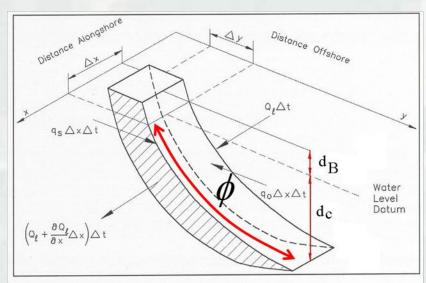
s : Subsidence Rate

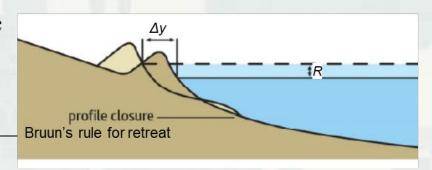
 $tan\beta$: beach slope

 $D_S = d_c + d_b(t)$: Total closure depth

Berm height varies with sea level change

$$d_b(t) = d_{b0} - (R+S)t$$





Longshore Sediment Transport - Energy Flux Method (CERC formula)

$$Q = H_b^2 C_{gb} \left(a_1 \sin 2\alpha_{bs} - a_2 \cos \alpha_{bs} \frac{\partial H_b}{\partial x} \right)$$

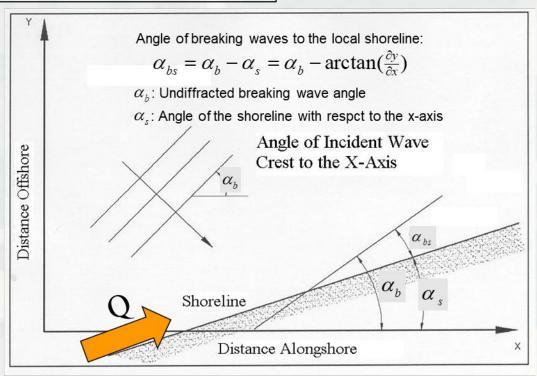
 H_b : Wave Height at breaker line

 C_{gb} : Group speed at breaker line

$$a_1 = \frac{K_1}{16(s-1)(1-p)1.416^{2.5}}$$

$$a_2 = \frac{K_2}{8(s-1)(1-p)\tan \beta 1.416^{2.5}}$$

 K_1, K_2 =empirical coefficients Typically, $0.5K_1 < K_2 < 1.5K_1$





Wave Diffraction around Structures

Wave Height (H_b) at P (at the lee side of a diffracting structure):

$$H_b = K_D(\theta_D, d_b) K_R(\theta_G, d_b) K_S(d_b) H_{tp}$$

 K_D = Diffraction coefficient at P

 K_R = Refraction coefficient at P

 $K_{\scriptscriptstyle S}$ = Shoaling coefficient at P

 d_b = water depth at P

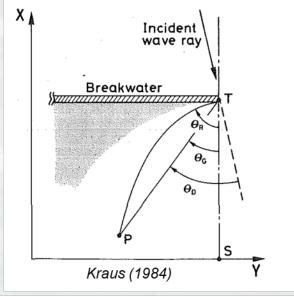
 H_{tp} = incident wave height at T

Kraus' Approximation of the 2-D diffraction solution (Kraus 1984)

$$K_D(\theta_D, d_b) = \sqrt{0.5 \left[1 + \tanh\left(\frac{S_{\text{max}}\theta_D}{W}\right)\right]}$$

$$W = 5.31 + 0.270S_{\text{max}} - 0.000103S_{\text{max}}^2$$

 $S_{
m max}$ = Maximum directional concentration parameter (10~75)



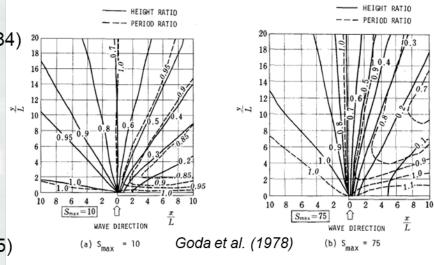
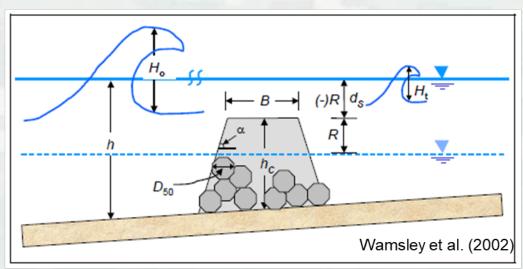
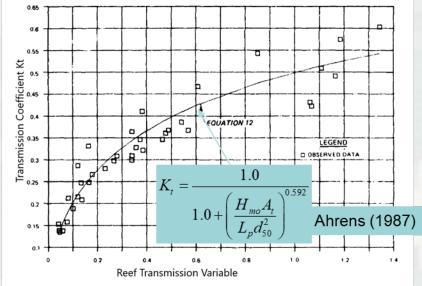


Fig. 6 Diffraction Diagrams of a Semi-Infinite Breakwater for Directional Random Waves of Normal Incidence

Wave Transmission through Breakwater





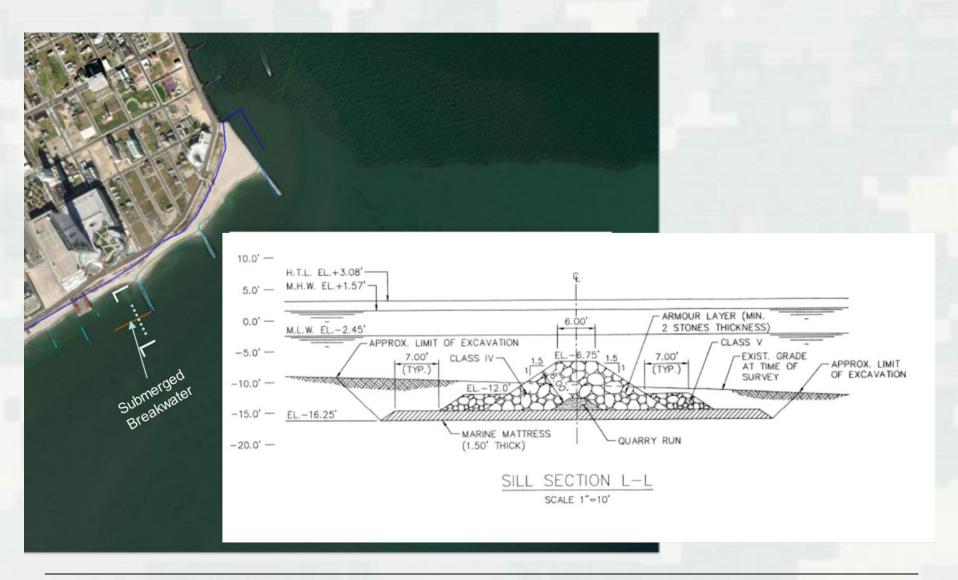
Wave transmission coefficient $K_t = H_t/H_o$

 K_t can be determined by means of the experimental studies on the features of structures;

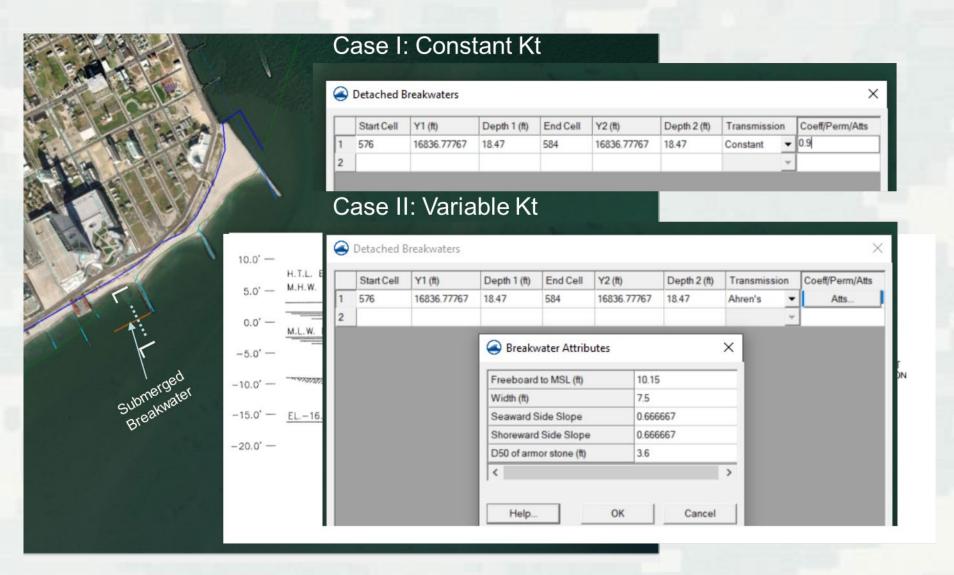
GenCade includes constant transmission and three equations for time-dependent wave transmission (Wamsley et al. 2002, CHETN-II-45):

- Ahrens (2001): for submerged and emergent (high) breakwaters
- Seabrook and Hall (1998): Only for submerged breakwater
- d'Angremond et al (1996): For deeply submerged and relatively high structures, the d'Angremond formulation is not recommended (0.75<R/H₀<0.5)

Wave Transmission in GenCade

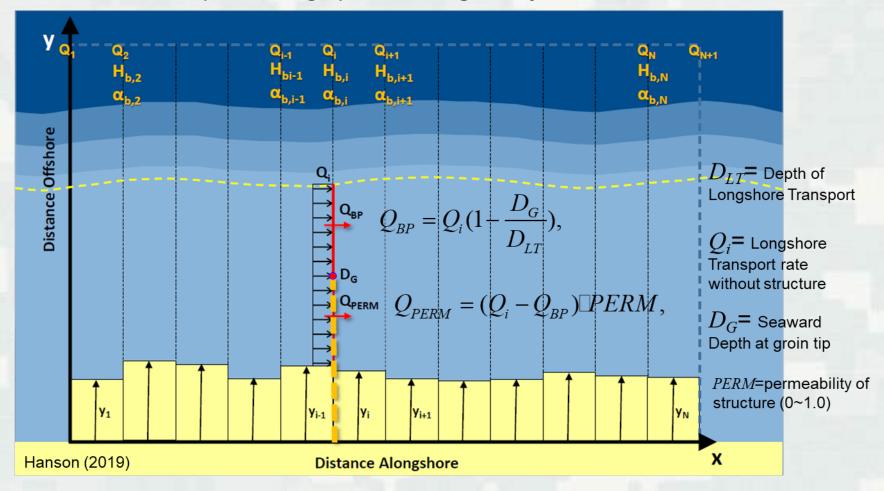


Wave Transmission in GenCade



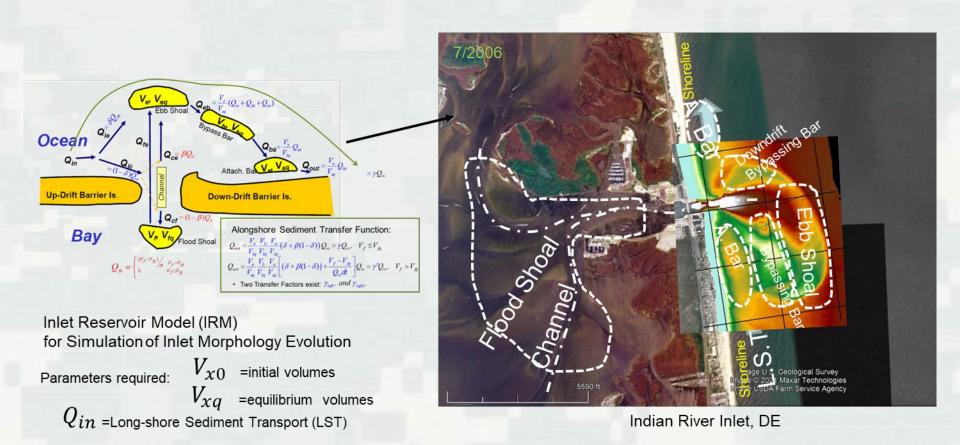
Sediment Bypassing through Groin

Transport through permeable groins/jetties



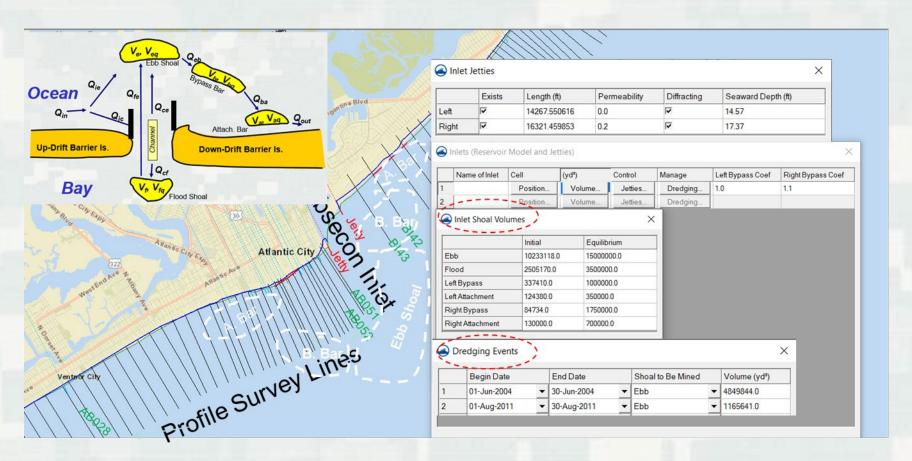
Flux at downdrift side: $Q_i^* = Q_{BP} + Q_{PERM} = Q_{BP} + (Q_i - Q_{BP}) \square PERM$

Inlet Reservoir Model and Inlet Morphologic Features

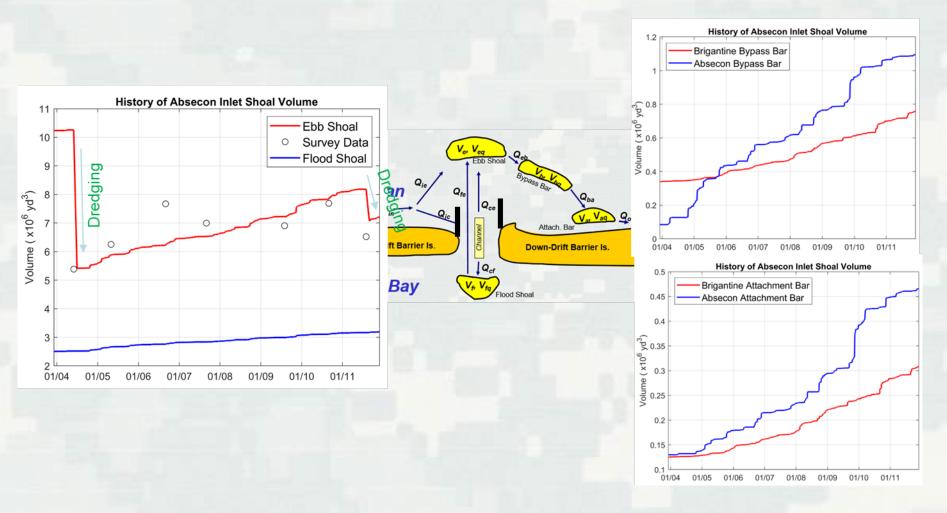


Dredging/Mining: ΔV from shoal or bar

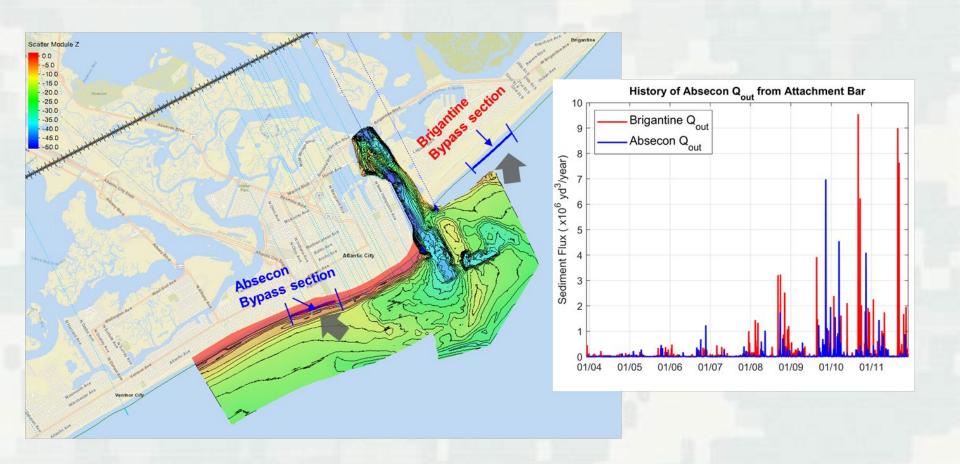
Inlet Reservoir Model (IRM) and Dredging: Example



Evolution of Inlet Shoals and Attachment Bars



Inlet Bypass Estimated by IRM model (2003-2011)

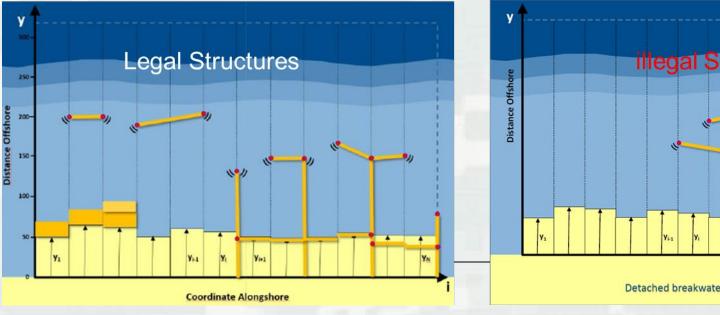


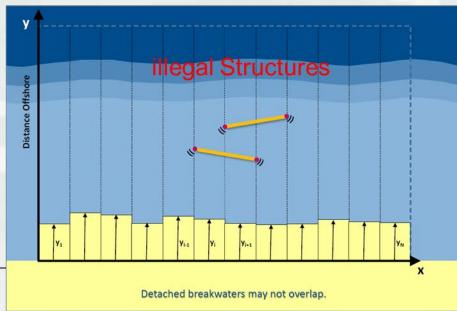
Data Requirements for Shoreline Modeling

- Shoreline Positions (beach profiles, topobathy, satellite imagery)
- Sediment properties (e.g., d50, slope)
- Wave data (height, period, direction)
- Tidal levels, SLR data
- History of engineering activities (structures, beachfills, mining/dredging, hydro survey data): locations, dates, dimensions, properties, etc
- Regional transport (longshore and cross-shore transport paths, sediment budget)
- Regional Geology (sediment source/sink, long-term trends in shoreline change, SLR, subsidence)
- Impact of extreme events (e.g., severe retreat by storms, structure failures, inlet close/opening, overtopping/overwash)
- Dune morphology changes
- Inlet data (hydro survey for mapping inlet morphology, dredging/mining, bypassing, etc.)

GenCade Limitations

- No wave reflection
- Some restrictions on structure shape, orientation, and placement
- No structure properties changing with time
- No profile changes with time (not yet)
- No dune change (Aeolian transport)
- No diffracting structure behind detached breakwater
- Restrictions on buried structures (no structures hidden into beach/berm)

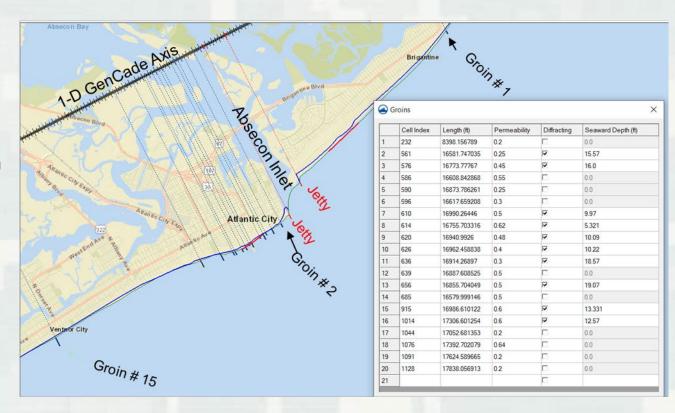




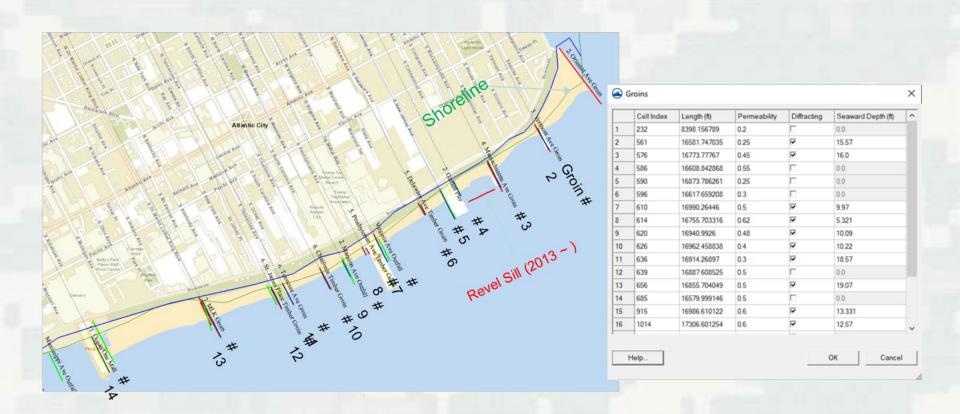
Parameters for Defining Structure's effect on LST

- Diffracting Structures:
 Groins, Piers
- Non-Diffracting Structures: Short groins, outfalls
- Seaward depth at the tip of groin is given from transect profile near the structure
- Length: the distance from the seaward tip to the GenCade 1-D Axis.
- Permeability (sand transmission capability through the structure):

calibrated



Calibrated Parameter for Structures



Display Simulated Shoreline on Historical Imagery (May-28-2008)

Laying simulated shoreline on historical images can further compare changes of shoreline curvature along the coast at different times and confirm the impact of structures on sediment transport. Google Earth Google Earth

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Recent Publications on GenCade

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- Ding, Yan, Kim, Sung-Chan, and Frey, Ashley, (2018). Probabilistic Shoreline Evolution Modeling in Response to Sea Level Changes, In Proceedings of the ASCE-EWRI 2018 Congress, June 3-7, 2018, Minneapolis, NM, 13p. (https://doi.org/10.1061/9780784481424.021)
- Ding, Y., Kim, S.-C., Frey, A.E., Permenter, R. L., and Styles, R. (2018). Probabilistic modeling of long-term shoreline changes in response to sea level rise and waves, In: Scour and Erosion IX: Proceedings of the 9th International Conference on Scour and Erosion, K.-C. Yeh Ed., pp203-211, Taylor & Francis Group, London, ISBN 978-0-367-07467-8
- Ding, Y., Kim, S.-C., Permenter, R.L., Styles, R., and Beck, T.M. (2019). Cross-shore sediment transport for modeling long-term shoreline changes in response to waves and sea level changes, In: Proceedings of Coastal Sediment 2019.
- Ding, Y., Styles, R., Kim, S.-C., Permenter, R.L., and Frey A.E. (2021a). Cross-shore sediment transport for modeling long-term shoreline evolution, J. Waterway, Port, Coastal, Ocean Eng., 2021, 147(4): 04021014, 25pp., DOI: 10.1061/(ASCE)WW.1943-5460.0000644
- Ding, Y., S. C. Kim, R. Permenter, R. Styles, and Gebert, J. A. (2021b). Simulations of Shoreline Changes along the Delaware Coast, ERDC/CHL TR-21-1, Vicksburg, MS: US Army Engineer Research and Development Center, http://dx.doi.org/10.21079/11681/39559, January 2021

Thank you for your attention!

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